



SESWA Regional Seminar BMP SELECTION CRITERIA

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Stantec



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Goals and Objectives

- Design Goals and Objectives
- Multi-Function BMPs
- Innovative Approaches

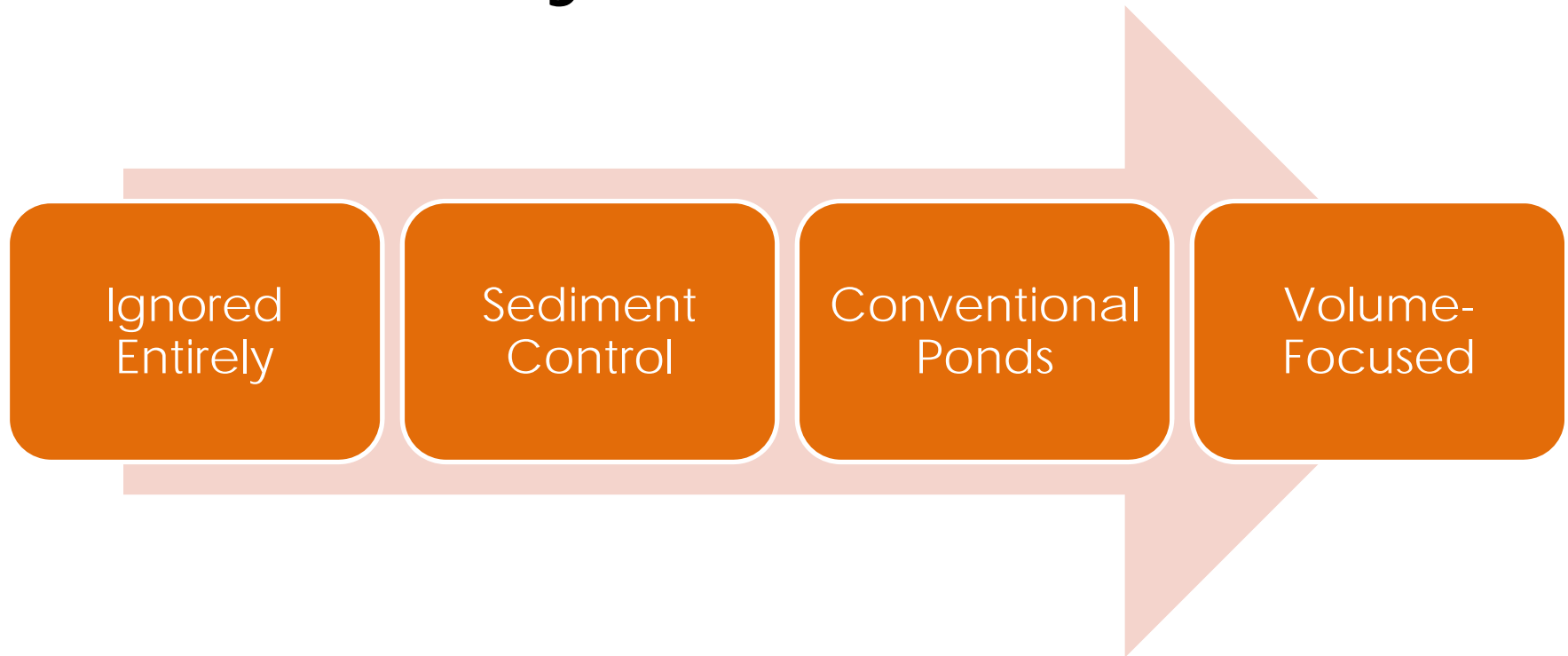
Background Questions:

- How many of you are **designers**?
- How many of you **review and approve plans**?
- How many of you **routinely see “LID” approaches** incorporated into projects?

BMP Design Goals and Objectives

- Site Drainage
- Erosion and Sediment Control
- Runoff Quality Management
- Receiving Channel Protection
- Flood Control

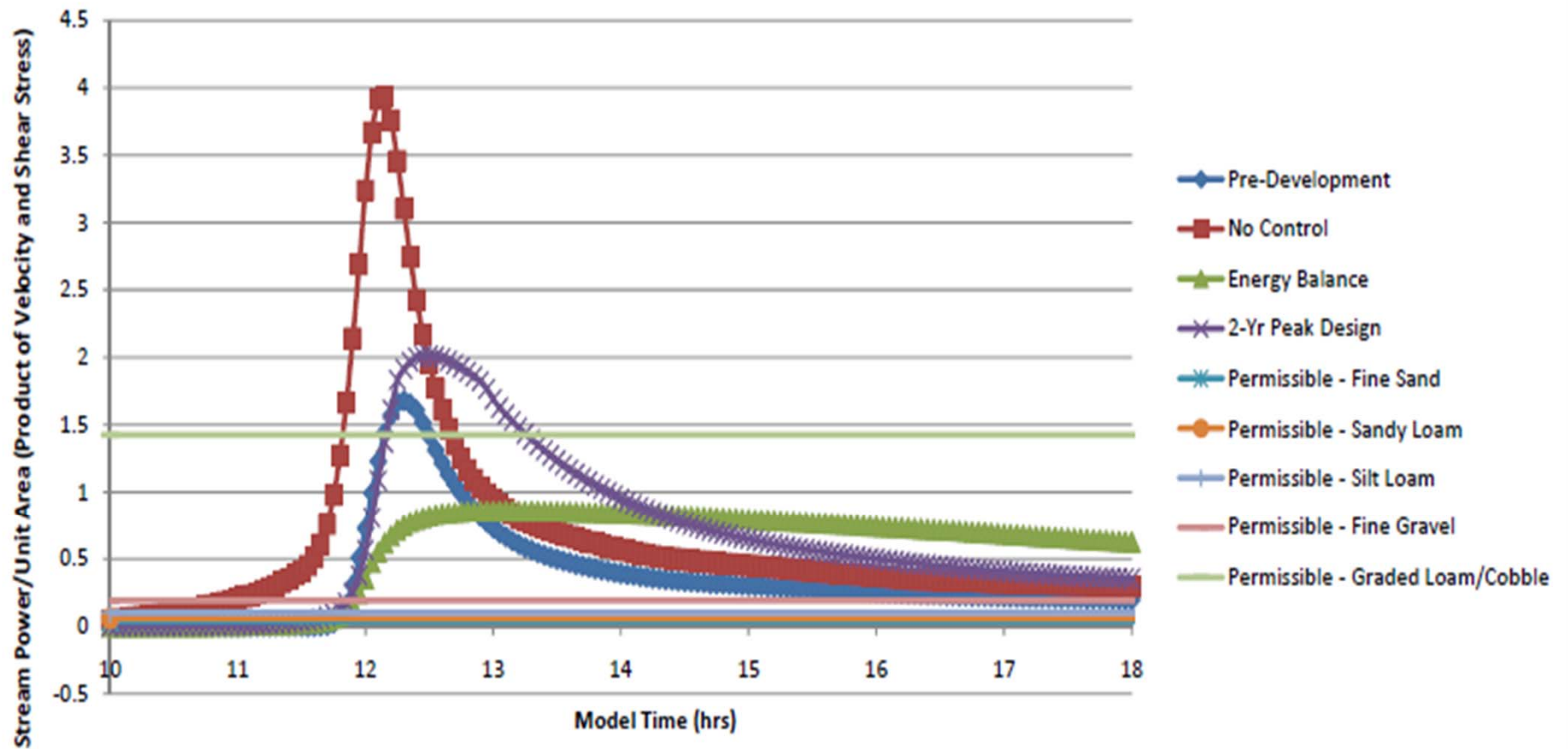
Evolution of Approaches: Runoff Quality



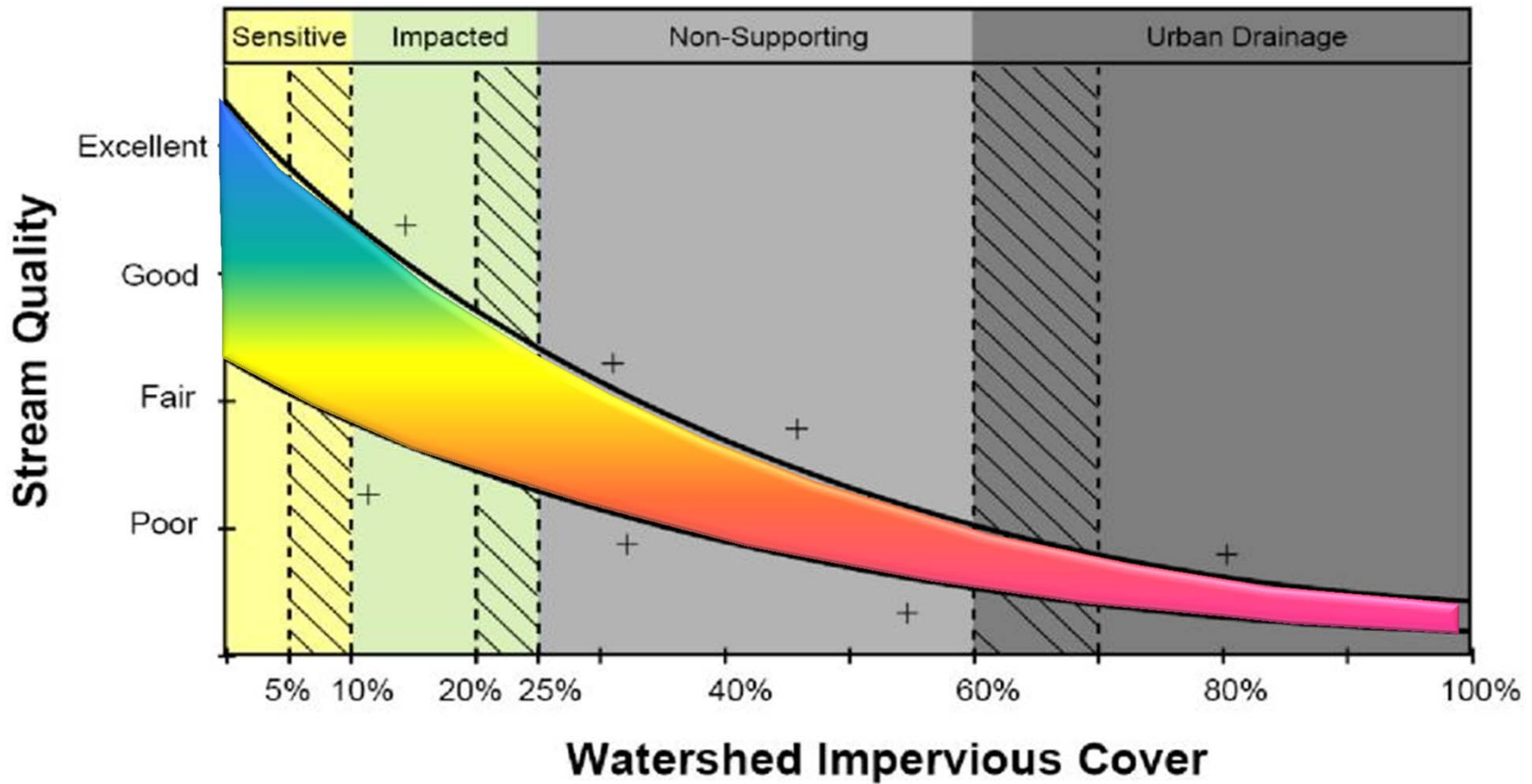


USGS, 2012

Power per Unit Area (lb/ft-sec) vs. Time Medium-Density Scenario



Impervious Cover Model



Managed Turf

- Documented impacts from turf management activities:
 - Fertilization;
 - Pest management;



Site Runoff Coefficients (Rv)¹

Cover	HSG A	HSG B	HSG C	HSG D
Forest/Open	0.02	0.03	0.04	0.05
Managed Turf / Disturbed Soil	0.15	0.20	0.22	0.25
Impervious Cover	0.95	0.95	0.95	0.95

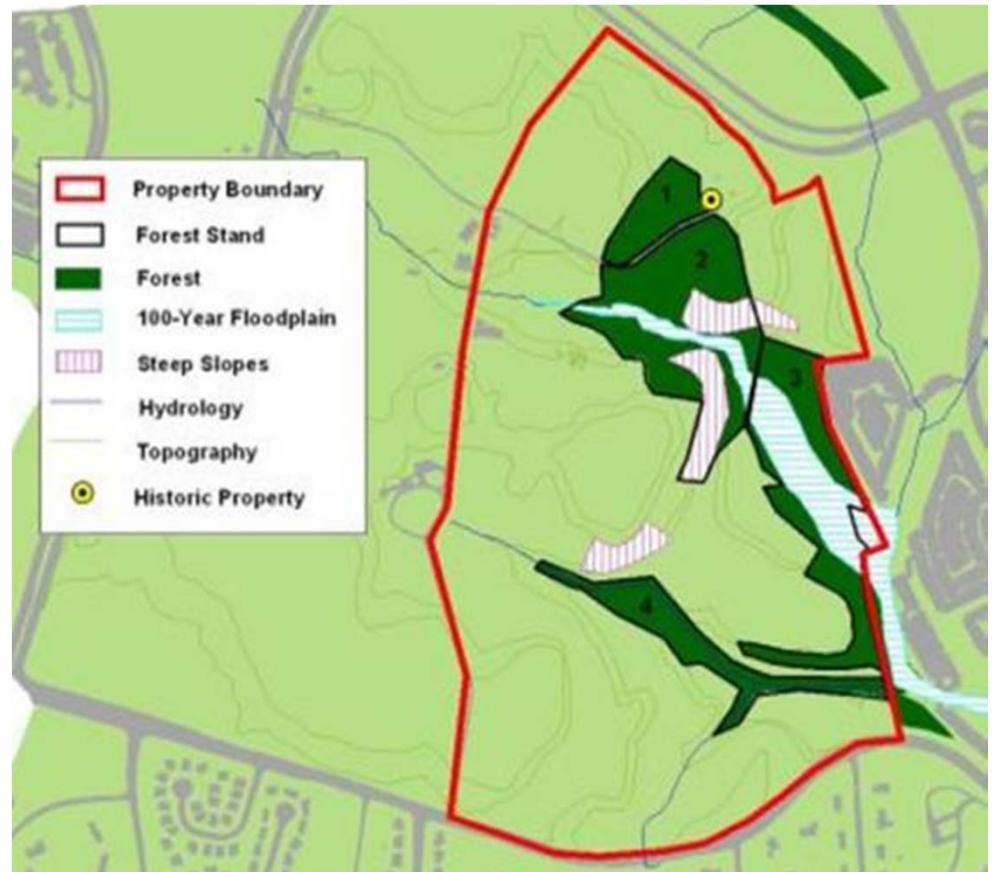
¹ Center for Watershed Protection – Technical Memorandum: The Runoff Reduction Method; 4/18/08

Pitt et al (2005), Lichter and Lindsey (1994), Schueler (2001a, 2001b, 1987), Legg et al (1996), Pitt et al (1999), and Capiella et al (2005)

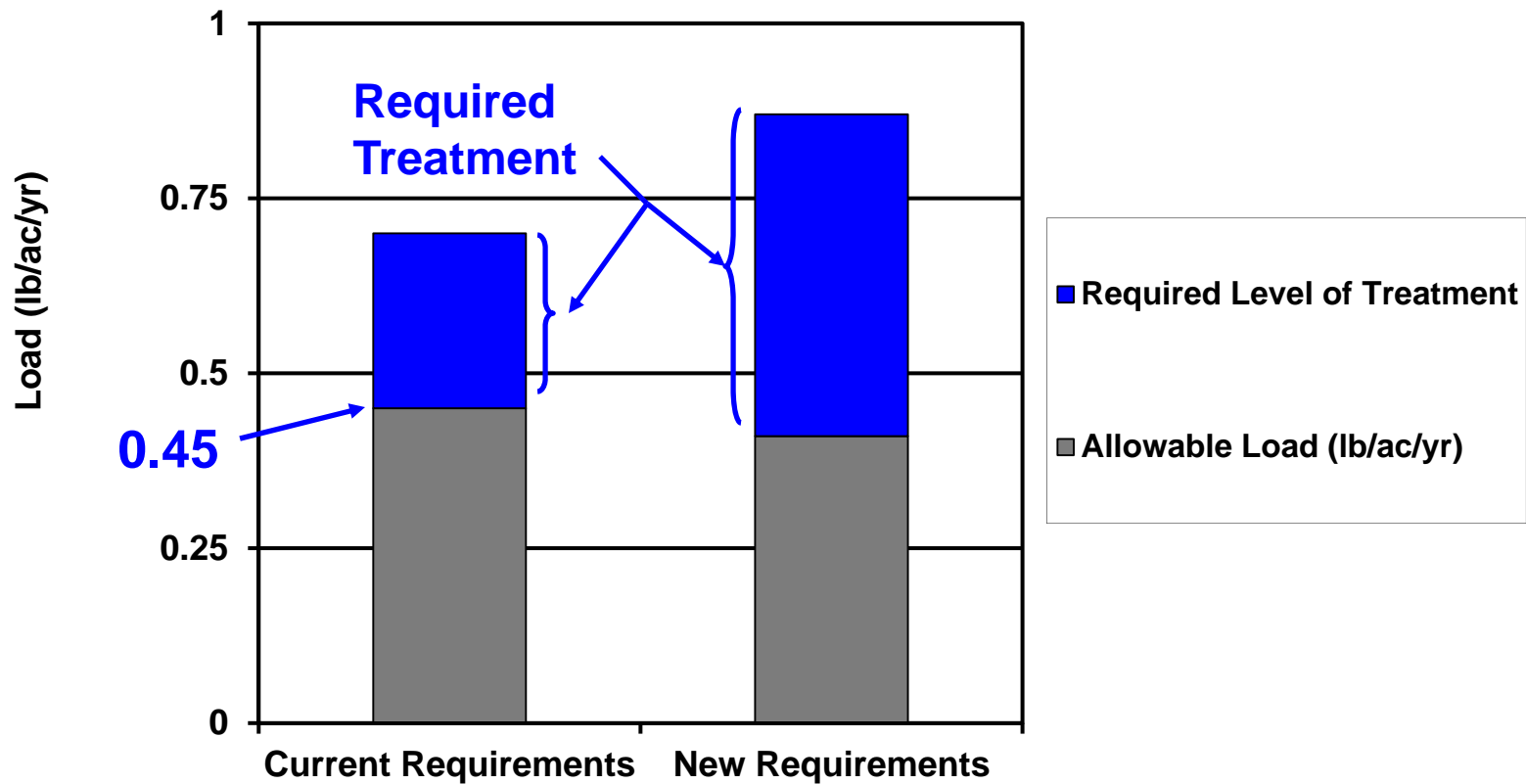
First Step in BMP Selection

Environmental Site Inventory & Assessment

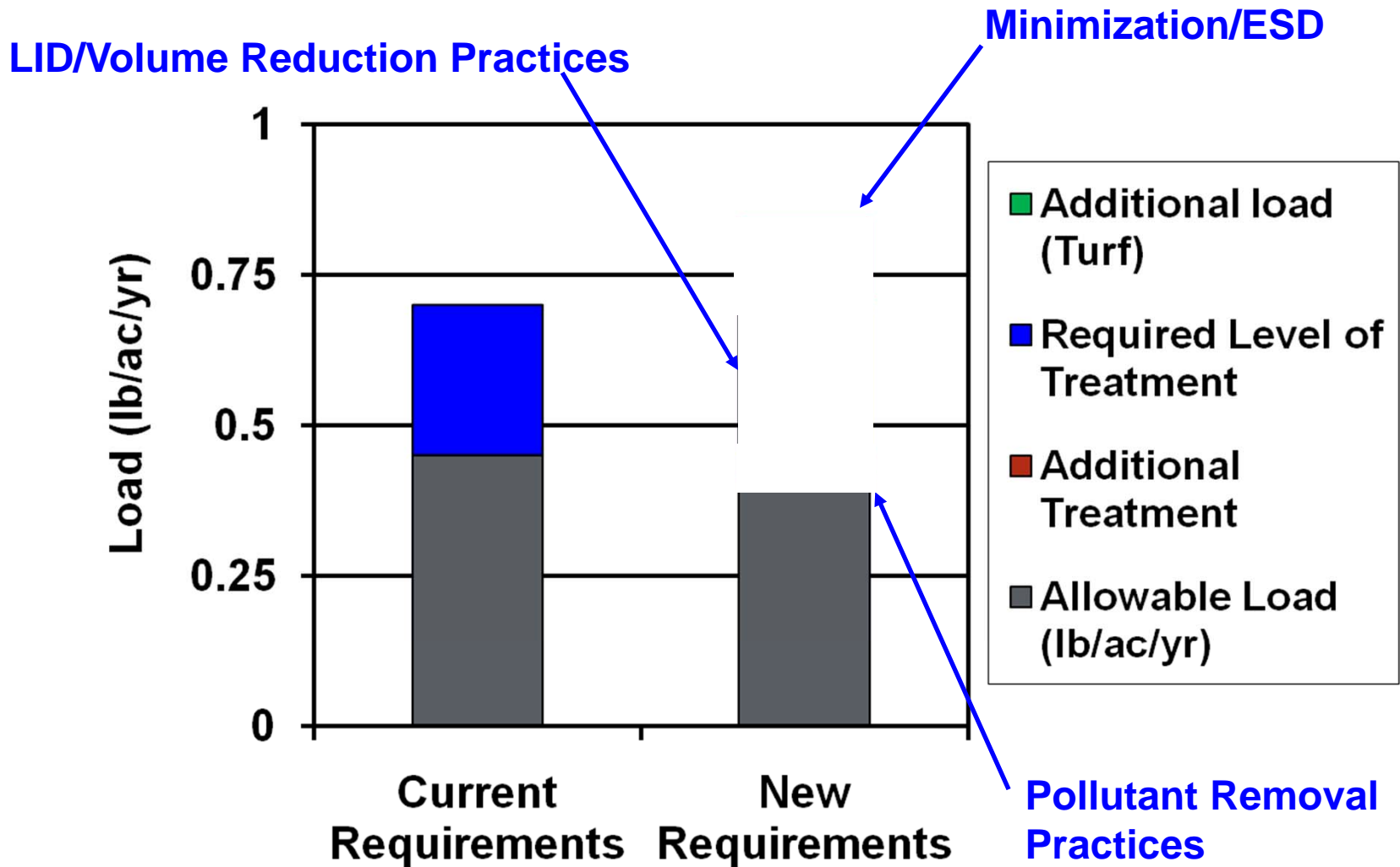
- Forest conservation
- Suitable soils
- Steep slopes
- Drainage
- Wetlands
- Zero-order streams
- Buffers
- Sensitive areas
- Limits of disturbance
- Computed nutrient loads & tv



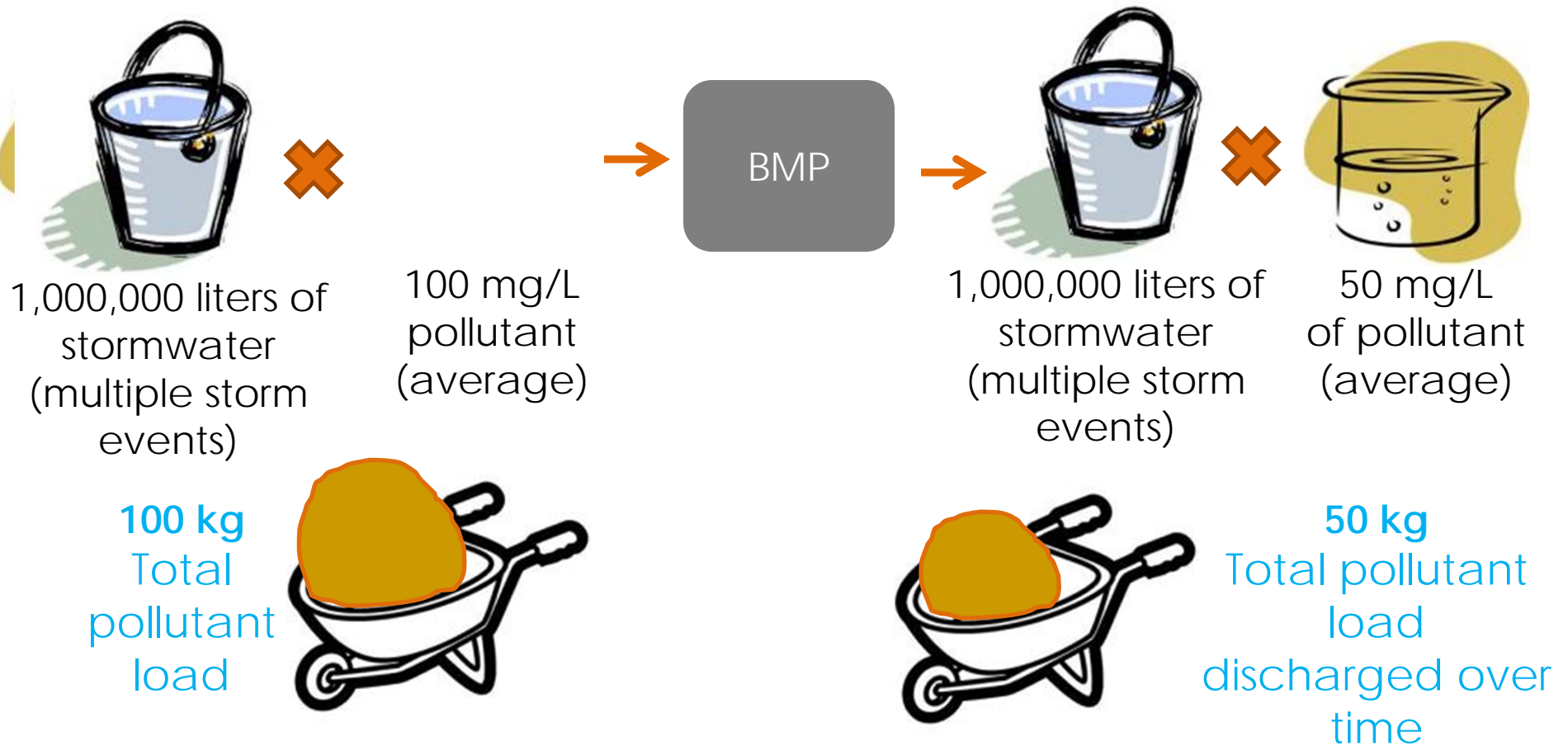
Water Quality - Treatment



Treatment Options

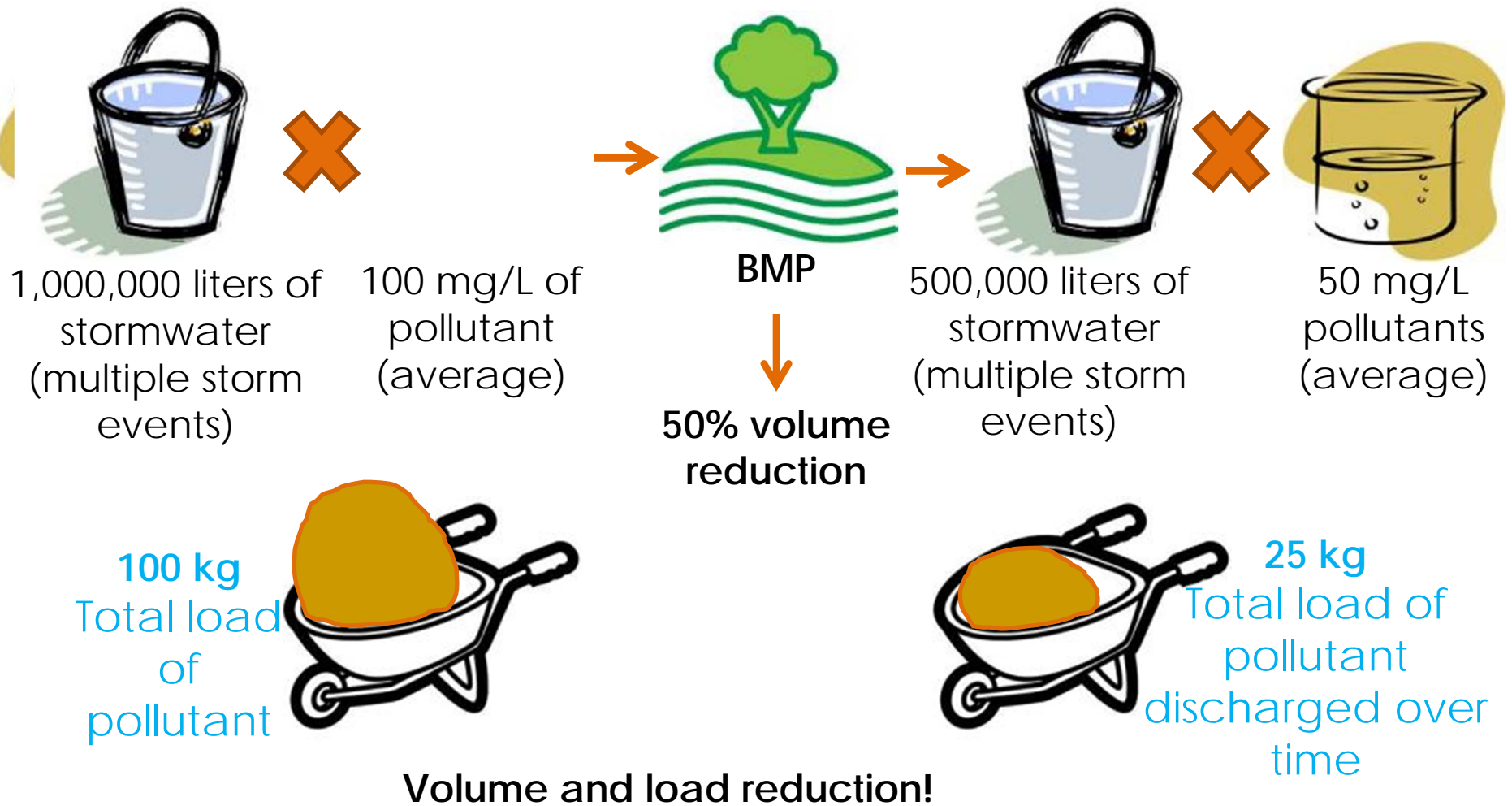


Traditional Approaches



No volume reduction, only load reduction

Volume Based Approaches

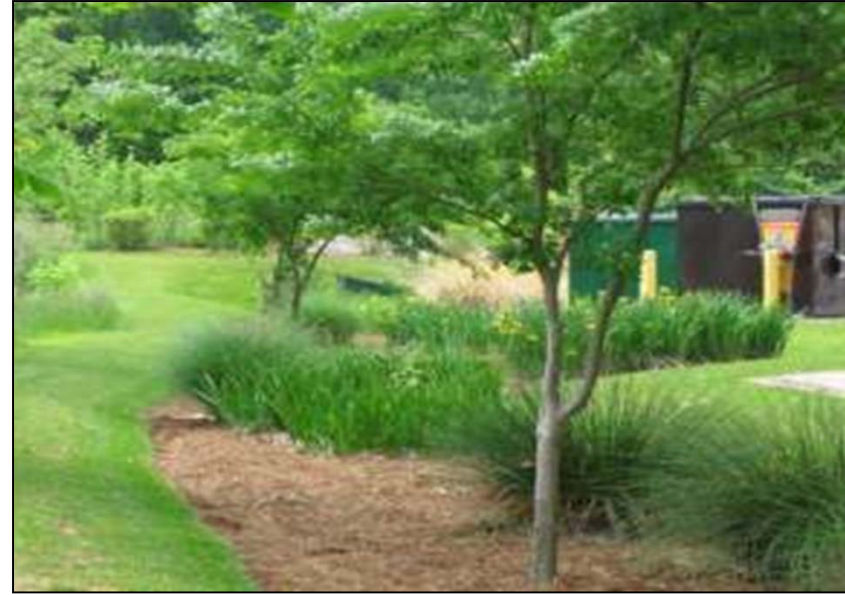


SOURCE: VA DEQ

Stormwater Practices Differ Sharply in Ability to Reduce Runoff Volume



Wet Ponds, ED Ponds and Constructed Wetlands and Filters Reduce Runoff Volumes by zero to 10%



Bioretention, Infiltration, Dry Swales, Soil Amendments, disconnection, and Related Practices Reduce Runoff Volumes by 50 to 90%

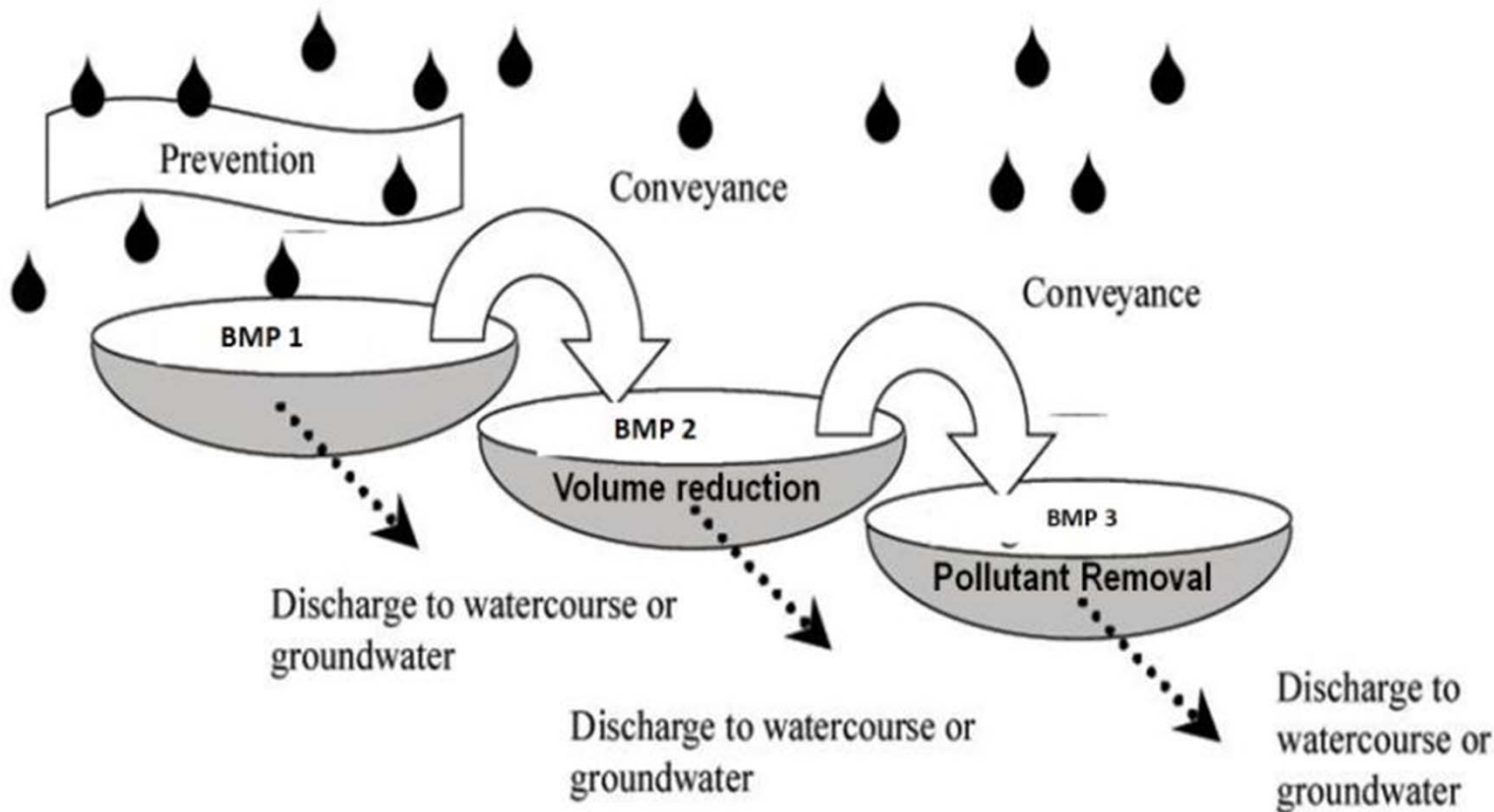
Practice	Design Level	Runoff Reduction	TN EMC Removal ³	TN Mass Load Removal	TP EMC Removal	TP Mass Load Removal ⁶
Rooftop Disconnect	1 ²	25 to 50 ¹	0	25 to 50 ¹	0	25 to 50 ¹
	No Level 2 Design					
Sheet Flow to Veg. Filter or Conserv. Open Space	1	50	0	50	0	50
	2 ⁵	50 to 75 ¹	0	50 to 75 ¹	0	50 to 75 ¹
Grass Channels	1	10 to 20 ¹	20	28 to 44 ¹	15	24 to 41 ¹
	No Level 2 Design					
Soil Compost Amendment	Can be used to Decrease Runoff Coefficient for Turf Cover at Site. See the design specs for Rooftop Disconnection, Sheet Flow to Vegetated Filter or Conserved Open Space, and Grass Channel					
Vegetated Roof	1	45	0	45	0	45
	2	60	0	60	0	60
Rainwater Harvesting	1	Up to 90 ^{3, 5}	0	Up to 90 ^{3, 5}	0	Up to 90 ^{3, 5}
	No Level 2 Design					
Permeable Pavement	1	45	25	59	25	59
	2	75	25	81	25	81
Infiltration Practices	1	50	15	57	25	63
	2	90	15	92	25	93
Bioretention Practices	1	40	40	64	25	55
	2	80	60	90	50	90
Urban Bioretention	1	40	40	64	25	55
	No Level 2 Design					
Dry Swales	1	40	25	55	20	52
	2	60	35	74	40	76
Wet Swales	1	0	25	25	20	20
	2	0	35	35	40	40
Filtering Practices	1	0	30	30	60	60
	2	0	45	45	65	65
Constructed Wetlands	1	0	25	25	50	50
	2	0	55	55	75	75
Wet Ponds	1	0	30 (20) ⁴	30 (20) ⁴	50 (45) ⁴	50 (45) ⁴
	2	0	40 (30) ⁴	40 (30) ⁴	75 (65) ⁴	75 (65) ⁴
Ext. Det. Ponds	1	0	10	10	15	15
	2	15	10	24	15	31

Multi-Function Practices

	Site Design	Runoff Reduction	Pollutant Removal
1. Rooftop Disconnection	✓	✓	
2. Filter Strip	✓	✓	
3. Grass Channel		✓	✓
4. Soil Amendments	✓*	✓	
5. Green Roof		✓	
6. Rain Tanks & Cisterns		✓	
7. Permeable Pavement		✓	✓
8. Infiltration		✓	✓
9. Bioretention		✓	✓
10. Dry Swales		✓	✓
12. Filtering Practices			✓
13. Constructed Wetlands			✓
14. Wet Ponds			✓
15. ED Ponds		✓	✓

BMP Treatment Train

Consider guidance to standardize *Process Diagrams* to track volume and load through complex treatment trains



Tools in the Toolbox

1. Impervious Disconnection
2. Sheetflow to Conservation Area/Filter Strip
3. Grass Channels
4. Soils Compost Amendments
5. Vegetated Roofs
6. Rainwater Harvesting
7. Permeable Pavement
8. Infiltration
9. Bioretention (including Urban Bioretention)
10. Dry Swales
11. Wet Swales
12. Filtering Practices
13. Constructed Wetlands
14. Wet Ponds
15. Dry Extended Detention Ponds

Rooftop/Impervious Area Disconnection

Simple Disconnection

Rainwater Harvesting & Cisterns;

Micro-Infiltration (dry wells);

Rain Gardens Urban Planter



Sheet Flow to a Vegetated Filter Strip or Conserved Open Space

Filter Strip & Open Space Design Criteria



Design Issue	Conserved Open Space	Vegetated Filter Strip
Soil and Vegetative Cover (Sections 6.1 and 6.2)	Undisturbed soils and native vegetation	Amended soils and dense turf cover or landscaped with herbaceous cover, shrubs, and trees
Overall Slope and length (parallel to the flow) (Section 5)	0.5% to 3% Slope – Minimum 35 ft length 3% to 6% Slope – Minimum 50 ft length The first 10 ft. of filter must be 2% or less in all cases ²	1% ¹ to 4% Slope – Minimum 35 ft. length 4% to 6% Slope – Minimum 50 ft. length 6% to 8% Slope – Minimum 65 ft. length The first 10 ft. of filter must be 2% or less in all cases
Contributing Area of Sheet Flow (Section 5)	Maximum flow length of 150 ft. from adjacent pervious areas; Maximum flow length of 75 ft. from adjacent impervious areas	
Level Spreader for dispersing Concentrated Flow (Section 6.3)	Length of ELS ⁶ Lip = 13 lin. ft. per each 1 cfs of inflow if area has 90% Cover ³ Length = 40 lin. ft. per 1 cfs for forested or re-forested Areas ⁴ (ELS ⁶ length = 13 lin.ft. min; 130 lin.ft. max.)	Length of ELS ⁶ Lip = 13 lin.ft. per each 1 cfs of inflow (13 lin.ft. min; 130 lin.ft. max.)
Construction Stage (Section 8)	Located outside the limits of disturbance and protected by ESC controls	Prevent soil compaction by heavy equipment
Typical Applications (Section 5)	Adjacent to stream or wetland buffer or forest conservation area	Treat small areas of IC (e.g., 5,000 sf) and/or turf-intensive land uses (sports fields, golf courses) close to source
Compost Amendments (Section 6.1)	No	Yes (B, C, and D soils) ⁵
Boundary Spreader (Section 6.3)	GD ⁶ at top of filter	GD ⁶ at top of filter PB ⁶ at toe of filter
<p>¹ A minimum of 1% is recommended to ensure positive drainage. ² For Conservation Areas with a varying slope, a pro-rated length may be computed only if the first 10 ft. is 2% or less. ³ Vegetative cover is described in Section 6.2. ⁴ Where the conserved open space is a mixture of native grasses, herbaceous cover and forest (or re-forested area), the length of the ELS⁶ Lip can be established by computing a weighted average of the lengths required for each vegetation type. Refer to Section 6.3 for design criteria ⁵ The plan approving authority may waive the requirement for compost amended soils for filter strips on B soils under certain conditions (see Section 6.1). ⁶ ELS = Engineered Level Spreader; GD = Gravel Diaphragm; PB = Permeable Berm.</p>		

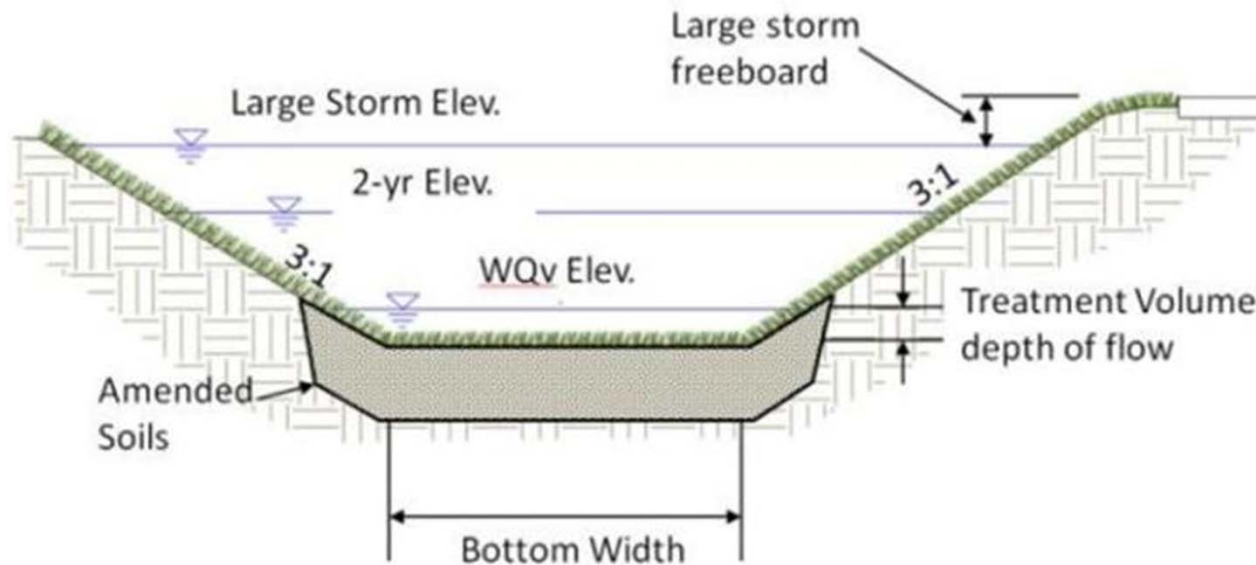
Soil Amendments



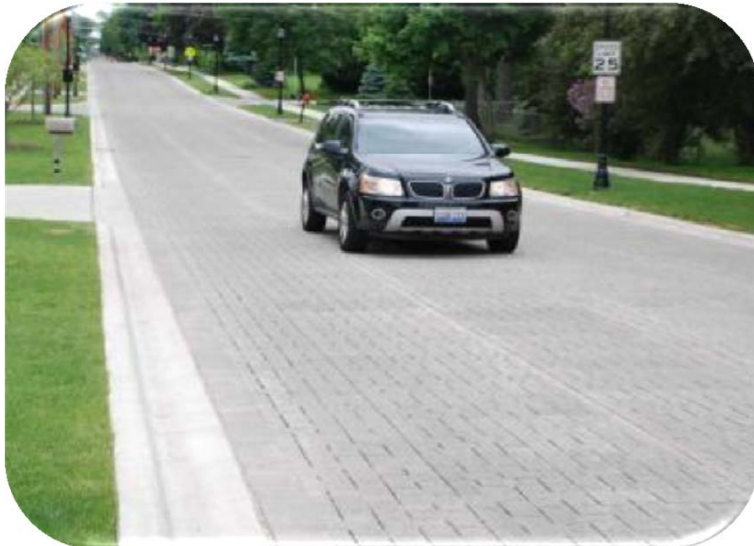
Grass Channels

Key Design Consideration: Soils

- Infiltration is greatest in HSG A soils;
- Infiltration gradually decreases in HSG B, C and D soils;
- HSG C and D soils lining the bottom of the Grass Channel can be amended to improve performance



Permeable Pavement



Stormwater Function	Level 1 Design	Level 2 Design
Annual Runoff Volume Reduction (RR)	45%	75%
Total Phosphorus (TP) EMC Reduction ¹ by BMP Treatment Process	25%	25%
Total Phosphorus (TP) Mass Load Removal	59%	81%
Total Nitrogen (TN) EMC Reduction ¹	25%	25%
Total Nitrogen (TN) Mass Load Removal	59%	81%
Channel Protection	<ul style="list-style-type: none"> • Use <u>VRRM</u> Compliance spreadsheet to calculate a Curve Number (CN) adjustment²; OR • Design extra storage in the stone underdrain layer and peak rate control structure (optional, as needed) to accommodate detention of larger storm volumes. 	
Flood Mitigation	Partial. May be able to design additional storage into the reservoir layer by adding perforated storage pipe or chambers.	

¹ Change in event mean concentration (EMC) through the practice. Actual nutrient mass load removed is the product of the removal rate and the runoff reduction rate (see Table 1 in the *Introduction to the New Virginia Stormwater Design Specifications*).

² NRCS TR-55 Runoff Equations 2-1 thru 2-5 and Figure 2-1 can be used to compute a curve number adjustment for larger storm events based on the retention storage provided by the practice(s).

Sources: CWP and CSN (2008) and CWP (2007)

Bioretention



Summary of Stormwater Functions ¹

Stormwater Function	Level 1 Design	Level 2 Design
Annual Runoff Volume Reduction (RR)	40%	80%
Total Phosphorus (TP) EMC Reduction ¹ by BMP Treatment Process	25%	50%
Total Phosphorus (TP) Mass Load Removal	55%	90%
Total Nitrogen (TN) EMC Reduction ¹ by BMP Treatment Process	40%	60%
Total Nitrogen (TN) Mass Load Removal	64%	90%
Channel and Flood Protection	<ul style="list-style-type: none"> • Use the Virginia Runoff Reduction Method (VRRM) Compliance Spreadsheet to calculate the Curve Number (CN) Adjustment OR • Design extra storage (optional; as needed) on the surface, in the engineered soil matrix, and in the stone/underdrain layer to accommodate a larger storm, and use NRCS TR-55 Runoff Equations² to compute the CN Adjustment. 	

¹ Change in event mean concentration (EMC) through the practice. Actual nutrient mass load removed is the product of the removal rate and the runoff reduction rate (see Table 1 in the *Introduction to the New Virginia Stormwater Design Specifications*).

² NRCS TR-55 Runoff Equations 2-1 thru 2-5 and Figure 2-1 can be used to compute a curve number adjustment for larger storm events based on the retention storage provided by the practice(s).

Sources: [CWP](#) and [CSN](#) (2008) and [CWP](#) (2007)

Stormwater Quantity Analysis Considering Volume



Treatment Volume & BMP Sizing

$$Tv_{BMP} = \frac{(P \times Rv_{composite} \times A)}{12}$$

Where:

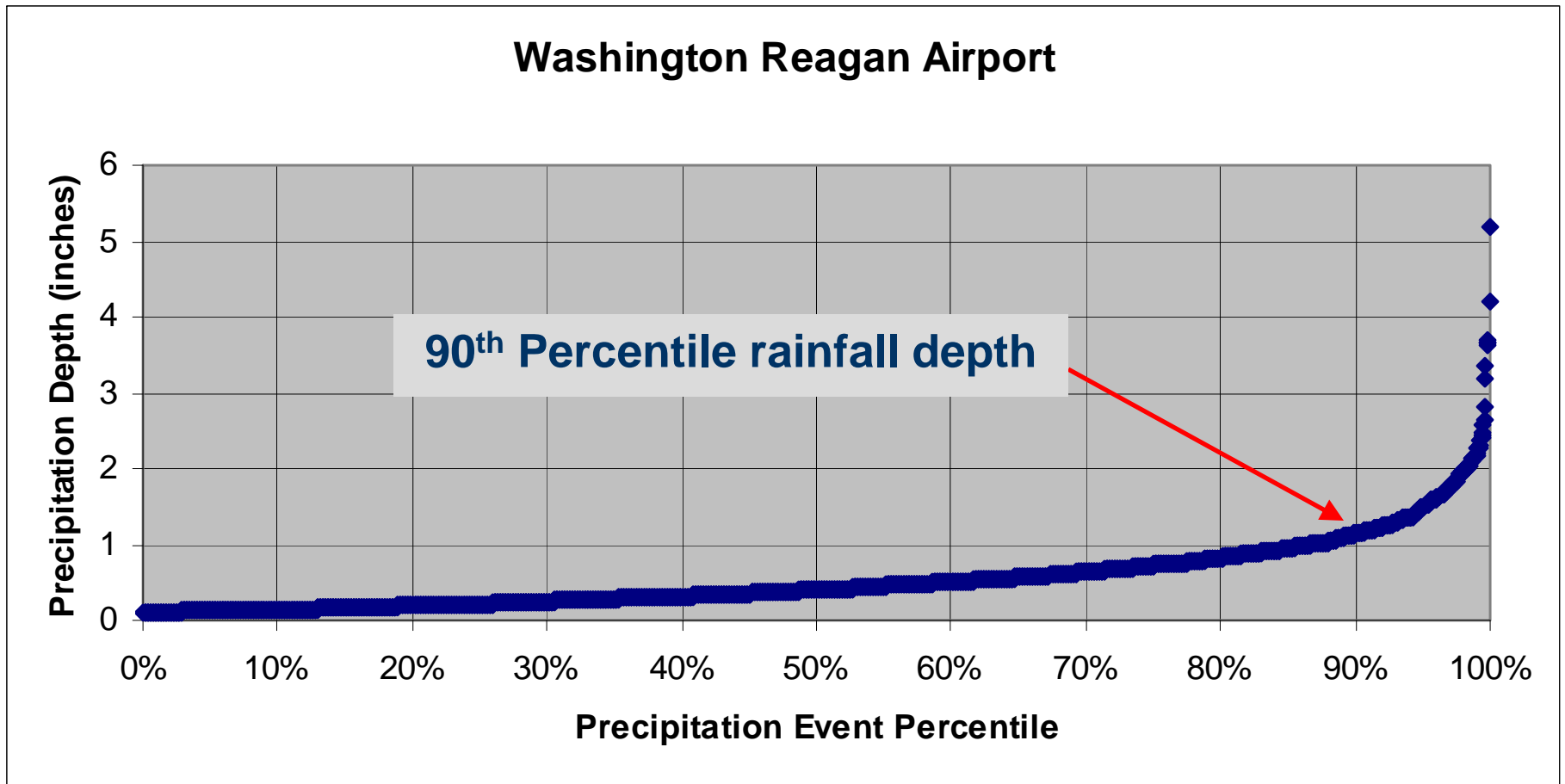
Tv_{BMP} = Design Treatment Volume from the contributing drainage area to the stormwater practice (does not include remaining runoff from upstream practices)

P = 90th Percentile rainfall depth = 1"

$Rv_{composite}$ = Composite runoff coefficient

A = Contributing drainage area to the stormwater practice.

Design Rainfall = 90th percentile rainfall
depth = 1”



1” annual average: Washington Reagan Airport, Richmond Airport,
Harrisonburg, Lynchburg, Bristol

Small Storm Hydrology

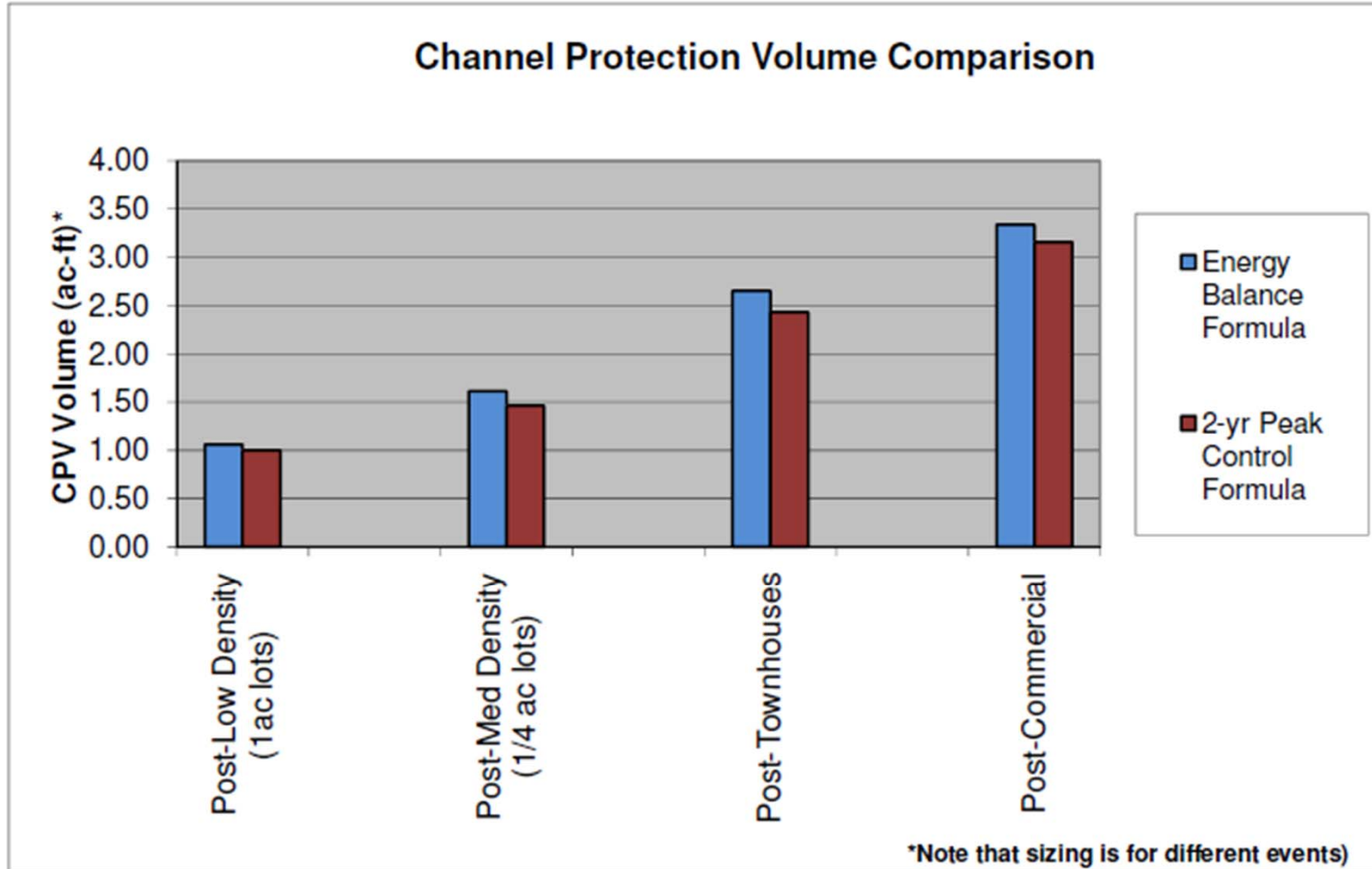
Volume Management
focused on small
storms

Focus is on minimizing
increases in stream
power and energy

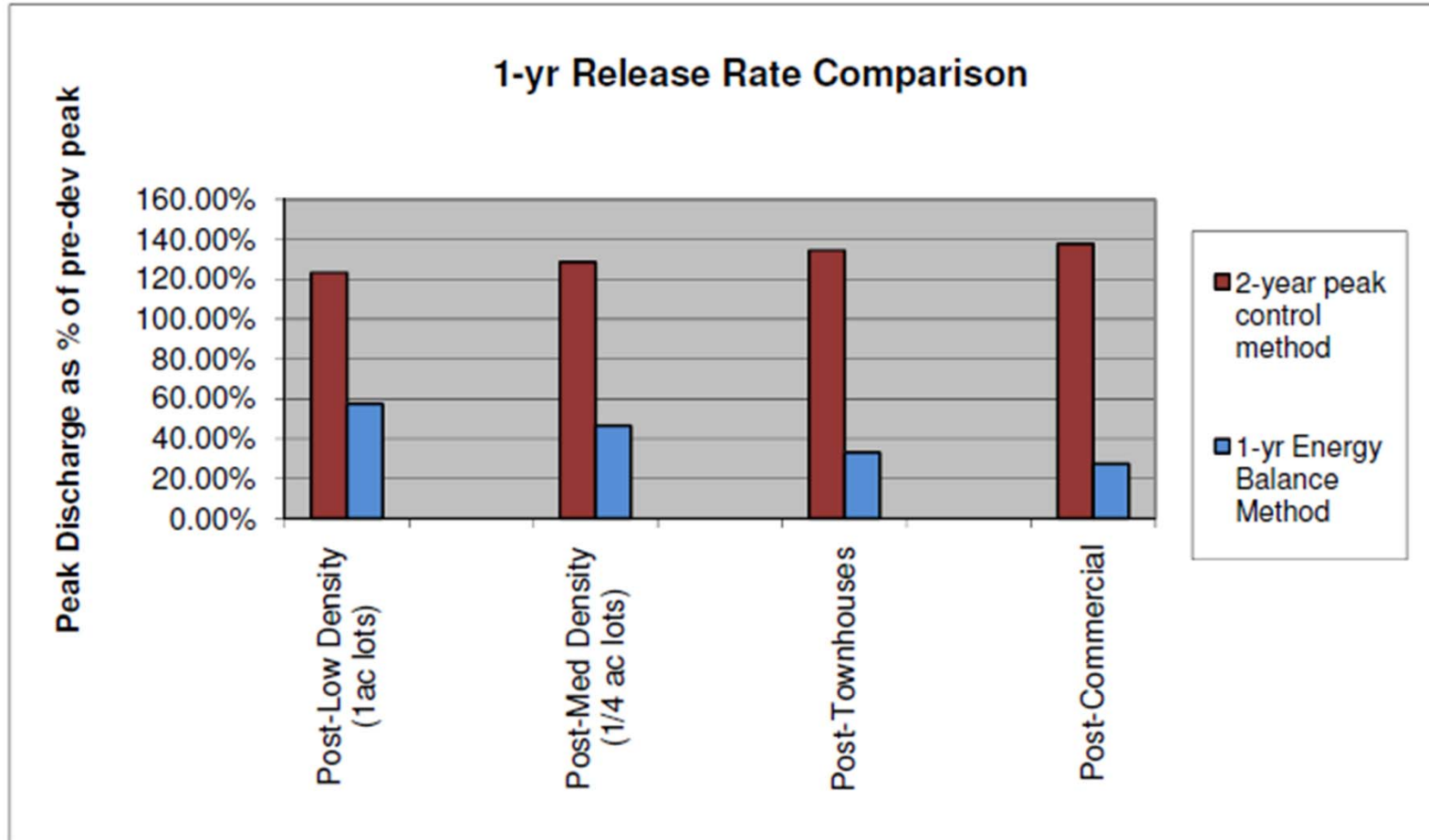
Replicating
depressional
storage and
abstraction from
natural watersheds



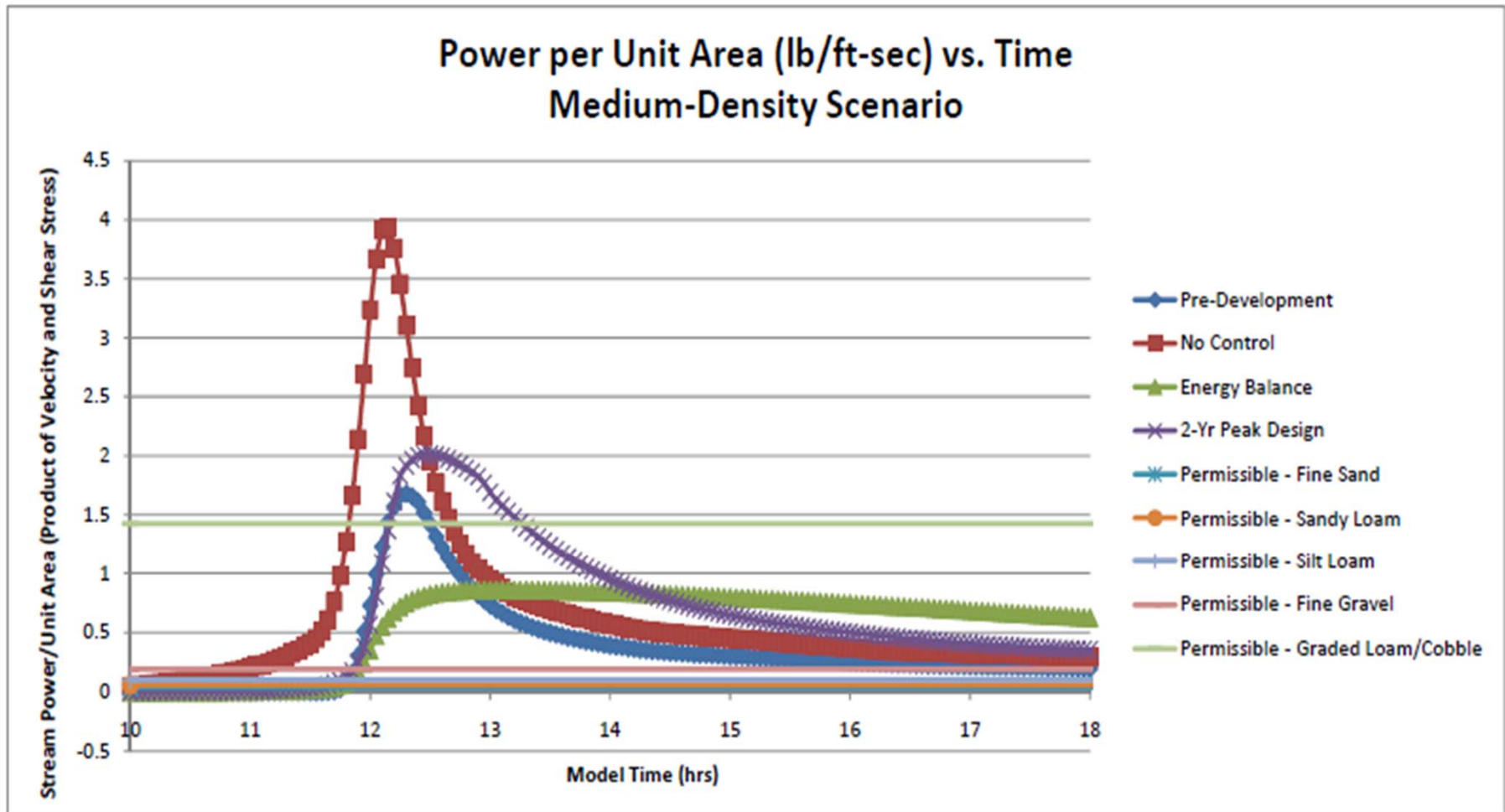
Sizing Comparison (+ 5-10%)



Release Rate Comparison



Stream Power Comparison



Challenge

Provide quantity “credit” for distributed retention practices

Avoid Complex routing/modeling

Allow designers to target volume as a primary metric (quantity and quality)

Various methods explored

Table 4. Review of Recent Research on Volumetric Runoff Reduction by LID Practices		
LID Practice	% Runoff Reduction	Reference
Bioretention	99	Dietz and Clausen (2006)
Bioretention	58	Seters et al (2006)
Bioretention	98	Rushton (2002)
Bioretention	50	Hunt et al (2006)
Bioretention	40 to 60	Smith and Hunt (2007)
Bioretention	75	Ballestro et al (2006)
Bioretention	80	Traver et al (2006)
Bioretention	73	Lloyd et al (2002)
Biofiltration Swale	98	Horner et al (2003)
Biofiltration Swale	94	Jefferies (2004)
Biofiltration Swale	46 to 54	Stagge (2006)
Permeable Pavement	75	Rushton (2002)
Permeable Pavement	99	Seters et al (2006)
Permeable Pavement	95 to 97	Traver et al (2006)
Permeable Pavement	60 to 90	Hunt and Lord (2006)
Permeable Pavement	50	Jefferies (2004)
Rainwater Harvesting	60 to 90	Coombes et al (2004)

Volume Reduction: Hydrograph Modification

Objective: Account for hydrologic effect of distributed retention storage;

Simplifying Assumptions:

- Assume retention is uniformly distributed if considering multiple features or sub-areas;
- Assume negligible discharge from under-drains (if any)

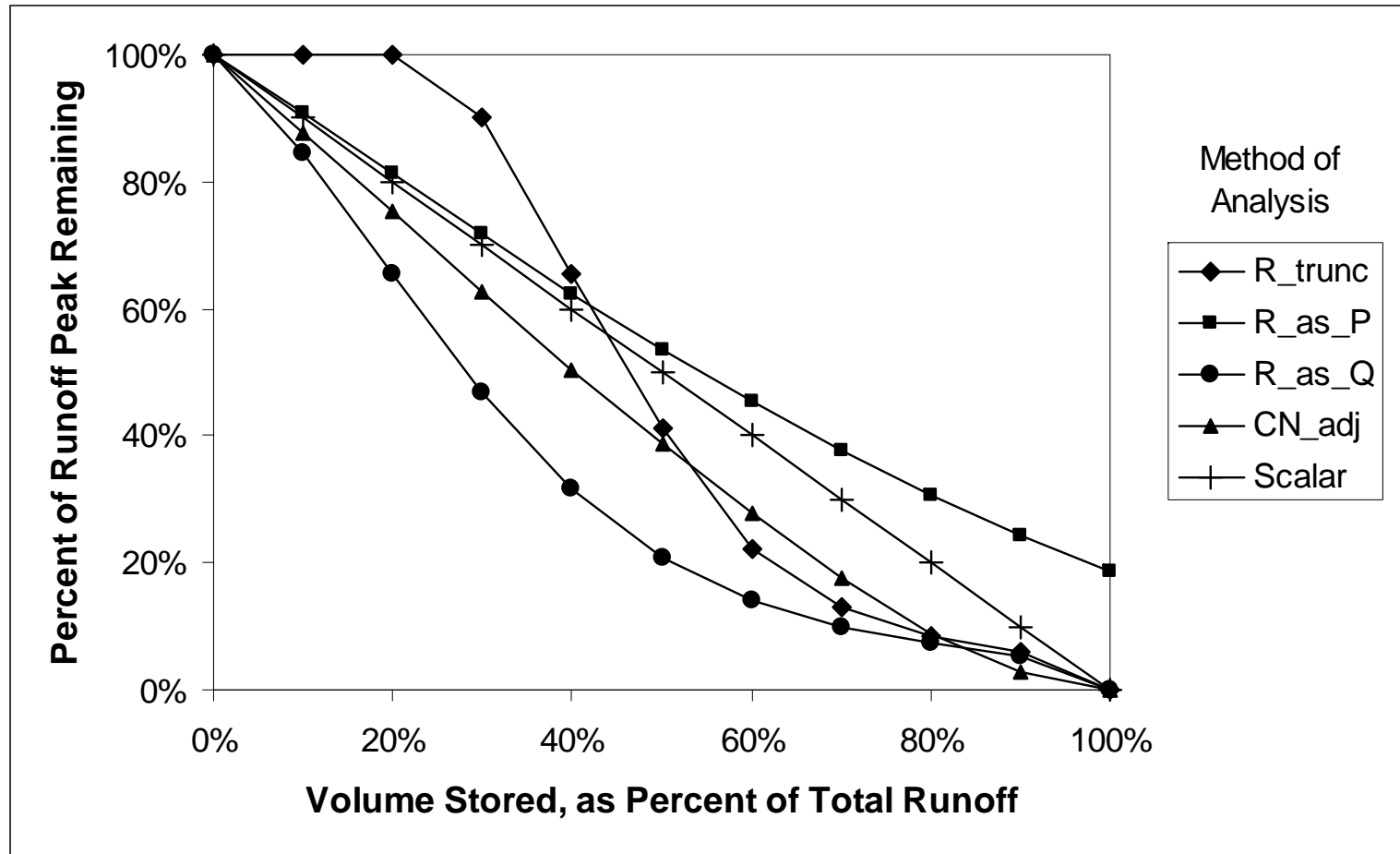
Volume Reduction: Hydrograph Modification

Methods Considered:

1. Hydrograph Truncation
2. Hydrograph Scalar Multiplication
3. Precipitation Adjustment
4. Runoff Adjustment
5. Curve Number Adjustment

Excerpted from work by Paul R. Koch, Ph.D., P.E.

5 Methods



Excerpted from work by Paul R. Koch, Ph.D., P.E.

Volume Reduction: Hydrograph Modification

Runoff Depth Equations (TR-55):

$$Q = \frac{(P - I_a)^2}{(P - I_a) + S}$$

Where:

Q = runoff depth (in)

P = precipitation depth (in)

S = potential maximum retention after runoff begins

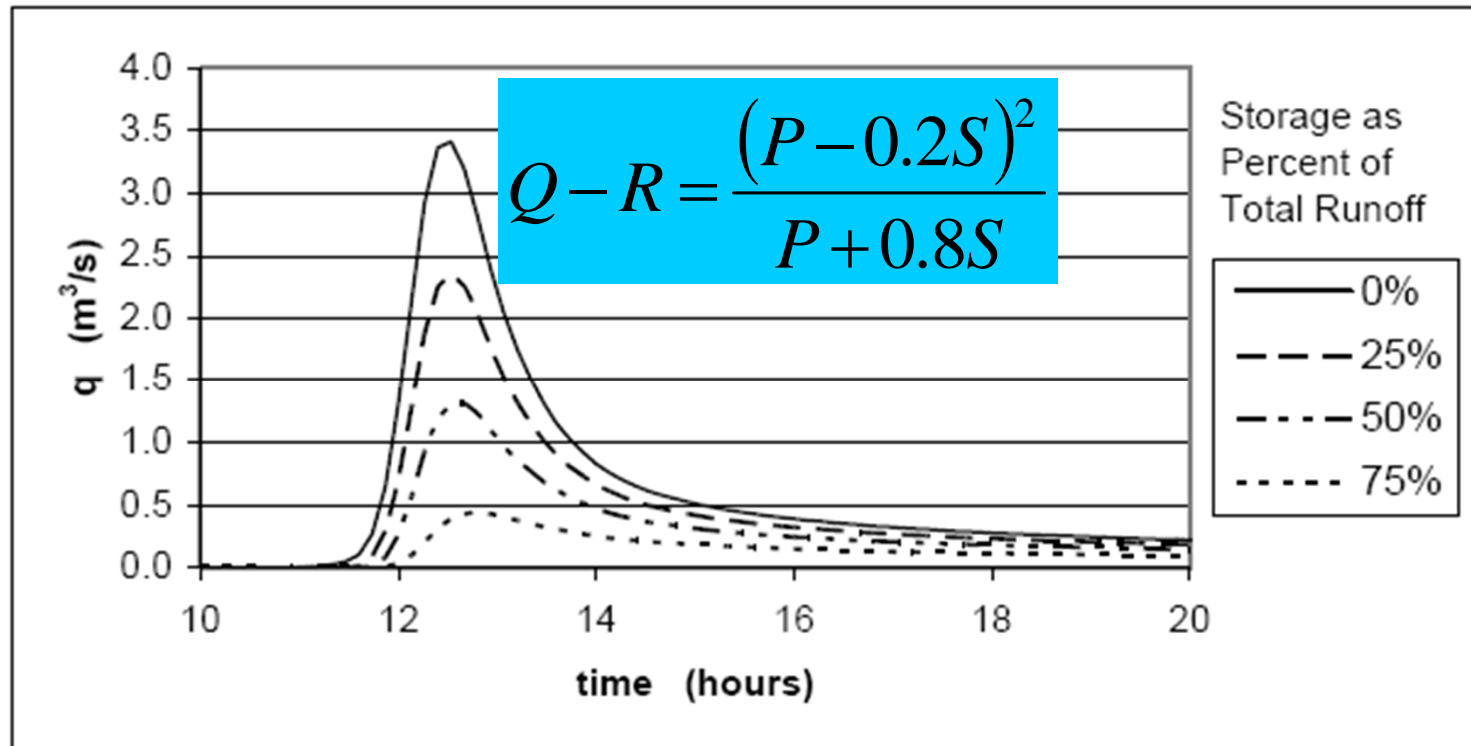
I_a = initial abstraction, volume that must be filled before runoff begins.

Additionally:

$$I_a = 0.2S$$

$$S = \frac{1000}{CN} - 10$$

Volume Reduction: Hydrograph Modification



NRCS Runoff depth formula solved for a new value of S , and then a revised CN value can be calculated from the revised S . No delay in the T_c is reflected, and the reduction is distributed across the entire storm, resulting in a conservative estimate of the peak discharge.

Effective CN Method

Original CN = 80

Adjusted
CN ~ 71

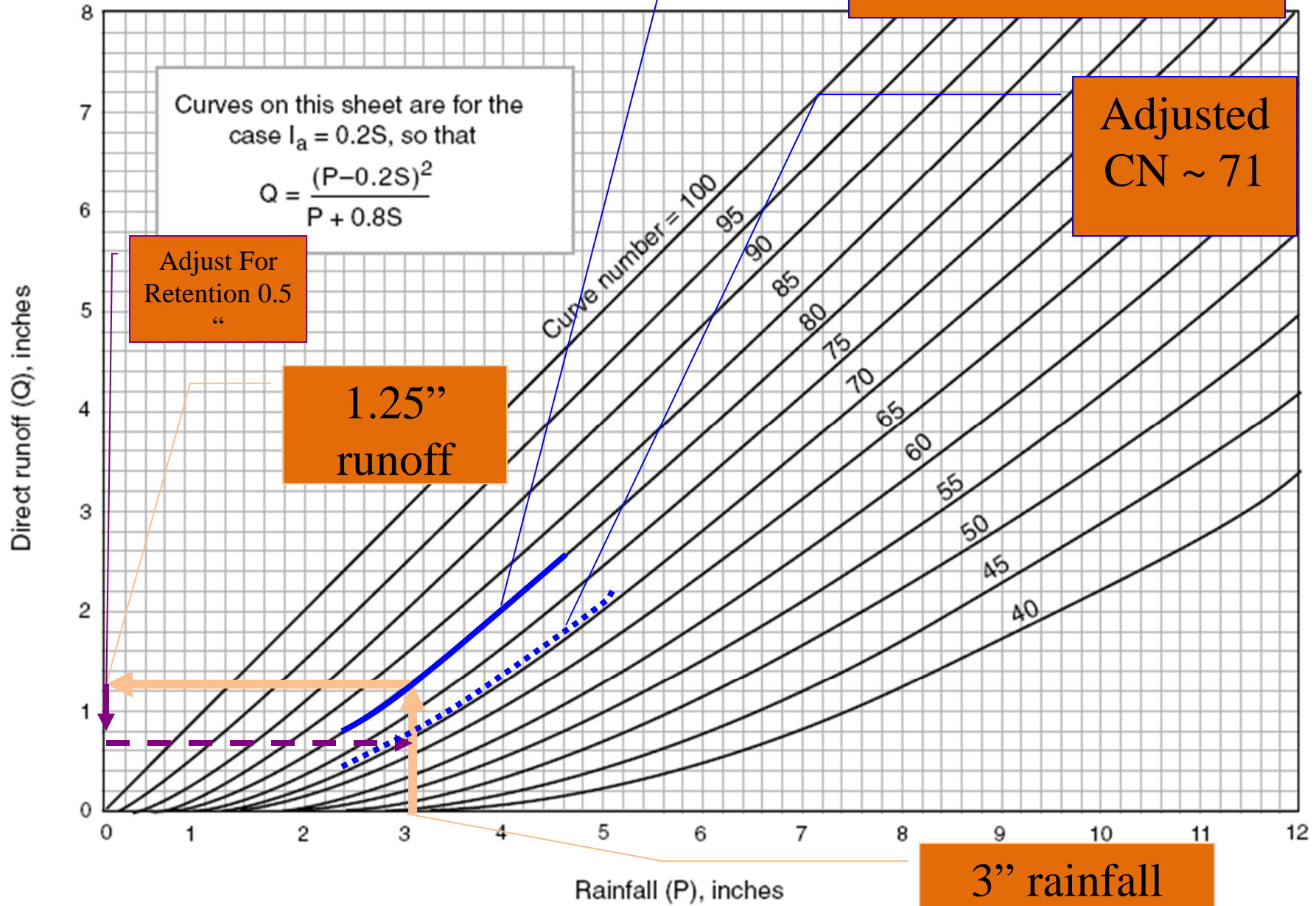
Curves on this sheet are for the
case $I_a = 0.2S$, so that

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S}$$

Adjust For
Retention 0.5
“

1.25”
runoff

3” rainfall



Site Parameters: ~39 Ac, Pre-CN=70, Post-CN=80, Lag time = 20 min(pre/post)

Design Approach	Original CN	Adjusted CN _(1-year)	Runoff (in)	Add'l Detention Storage Req'd (ft ³)	Treatment Approach
Conventional Design	80	80	1.25	73000	Treat with 2 acre wet pond
LID Practices	80	75	0.95	37000	Bioretention, Grassed Channels w/ soil amendments
Better Site Design with LID	80	73	0.85	28000	Reduce Impervious Cover, Reduce Turf Acreage + above
Pre-Development	70	N/A	0.71	N/A	N/A

**Recurrence
Interval**

	1-yr	2-yr	10-yr
Total Rainfall (in)	2.6	3.5	5.6
Pre-dev CN	70	70	70
Pre-Dev Runoff (in)	0.50	1.01	2.49
Post-Dev CN	80	80	80
Runoff (in.)	0.96	1.64	3.43
Runoff Reduction Vol. (in.)	0.27	0.27	0.27
Net Runoff (w/ RRM, in)	0.69	1.37	3.16
CN Adjusted for RRM	75	76	77
% Redux In Runoff Volume	28.0%	16.4%	7.9%

Innovative/Emerging Approaches

Issues in Urbanized Areas

- Historical/Legacy Urbanization
- Minimal Stream Functions and Values
- Surface Practices Cost Prohibitive/Innefectual
- Forces Watershed-based Approaches

Stream Restoration

- Reduces Nutrients and Sediments
- Protect Property and Infrastructure
- Improves Ecology
- Non-land Consumptive



Nutrient and Sediment Loadings
are dramatically affected by
urban stream restoration

Stream and Shoreline Restoration
can affect multiple objectives:

- Protect Property & Infrastructure
- Improve Flood Conveyance
- Ecological Functions & Values
- Compatible with Park/Trail systems
- Not (as) Land-Consumptive
- Enhance Aesthetics

Stream Restoration/Stabilization

- Treatment Mechanisms:
 - “Pass through” physical, chemical, and biological treatment of the improved natural system
 - *Research ongoing, not covered herein*
 - Reduction in bank erosion = reduction in nutrients associated with the bank sediment
 - *Standard Methodologies under development*

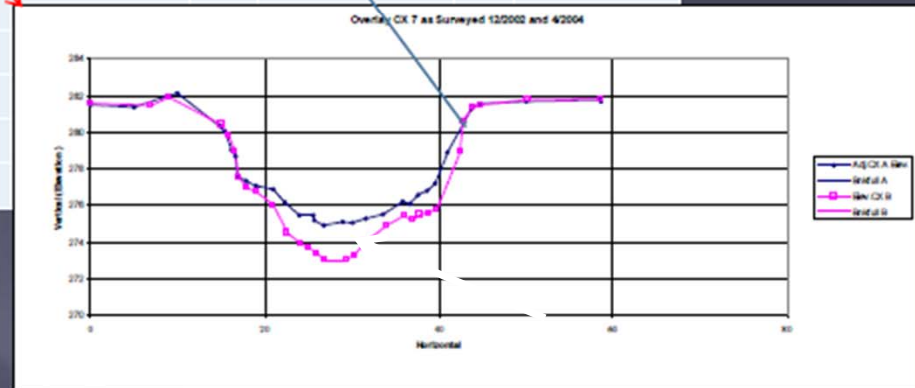
Stream Restoration/Stabilization Nutrient Reduction

- Detailed Studies:
 - Sediment Transport Modeling
 - Physical Sampling
- Simpler, more practical methods
 - BANCS Method (Rosgen)
 - Maryland Guidance
 - City of Baltimore Dept. of Public Works
 - “Sediment Wedge” Calculations
 - Measured Historical Bank Erosion Rates
 - Predictive Geomorphology (Channel Evolution Model)
 - Stable Channel Hydraulic Analysis



Data for local curve for Stony Run

Stream Bank	NBSS	Avg. Erosion	NBSS	BEHI
No.	Rating	Rate	Rating	Rating
1-1L	2	0.1	Moderate	High
1-1U	4	0.156	Very High	Moderate
1-2U	5	0.343	Extreme	High
3-1U	4	0.182	Very High	High
4-1U	4	0.515	Very High	High
4-2U	5	0.206	Extreme	Moderate
5-1U	4	0.171	Very High	High
3-1L	0.01	0.01	Very Low	Moderate
4-1L	4	0.48	Very High	High
5-1L	5			
5-2L	4			
5-3L	4			
6-2L	4			
7-1L	5			



Source: CWP: Urban Stream Restoration Expert Panel, 2012

Stream Erosion

Typical Bank-line
Sediment Conc.
btw: 100-200
mg/kg TP

Scale of the
problem can be
staggering (1000s
of tons of
sediment/yr from
degraded urban
stream channels)



Pre-Restoration

CBWM reflects up to
600 lb/ac of sediment
generated by the most
urbanized watersheds

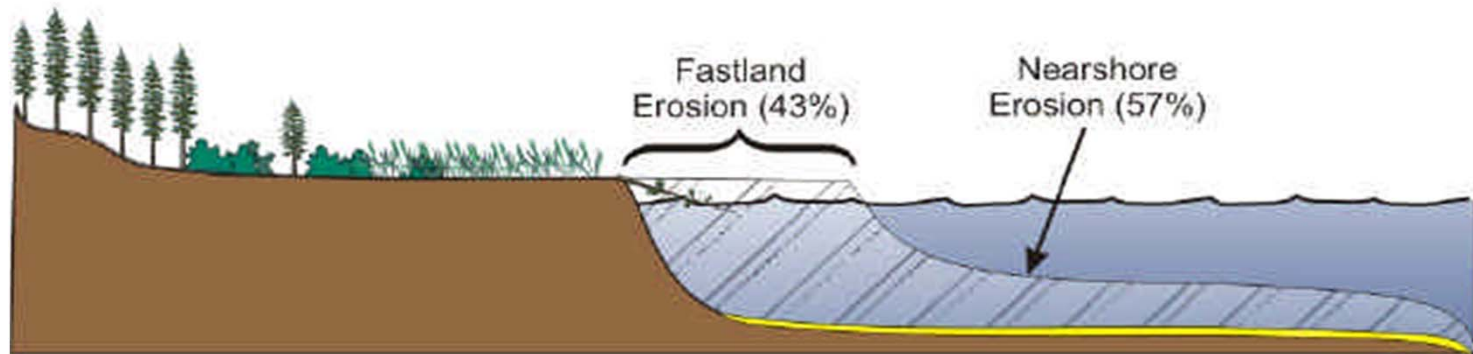
Shoreline Nutrient Reductions

Similar to Stream Restoration

Shoreline Erosion = Sediment Load = Nutrient Load

- Sediment from Bank and Nearshore Material
- Nutrients Attached to Sediment
 - Nitrogen
 - Phosphorus

Shoreline Stabilization Stops the Erosion
Sediment & Nutrient “Removal” Credit



Source: Maryland Geological Survey/Chesapeake Bay Program (modified from USACE, 1990)

Examples of Shoreline Stabilization Practices

Conventional

- Bulkheads
- Seawalls
- Riprap Revetments

➤ Living Shorelines

- Marsh Sills
- Nearshore Breakwaters with Beach Nourishment

No one solution is appropriate for all cases – site specific



Source: Google Imagery (www.googlemaps.com)

Shoreline Nutrient Reductions: Past Research

- Numerous Studies from 1970s – Present
 - USACE
 - Virginia Institute of Marine Science (VIMS)
 - Virginia Dept. of Conservation and Recreation (DCR)
 - Chesapeake Bay Program
 - Maryland Dept. of the Environment



Agricultural Nutrient Offsets

Significant Federal Support at
EPA/USDA

Agricultural Trading Guidance
and Support Available

Offset Credit Generation
generally constrained to
Land Conversion

Service area defined (similar
to
mitigation banking)



Trading Nutrient Reductions from Nonpoint
Source Best Management Practices in the
Chesapeake Bay Watershed: Guidance for
Agricultural Landowners and Your Potential
Trading Partners



Non-traditional Surface Water Quality Offsets

Land/Mine Reclamation

Pollution Abatement

Nutrient Management

Large scale ecological
improvements (constructed/
created wetlands)



Questions?

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